

Direct parametrisation of invariant manifold with shell finite element: nonlinear dynamics of thin structures using reduced order modeling

17-18 octobre 2024

Journées annuelles du GdR EX-MODELI | Lyon

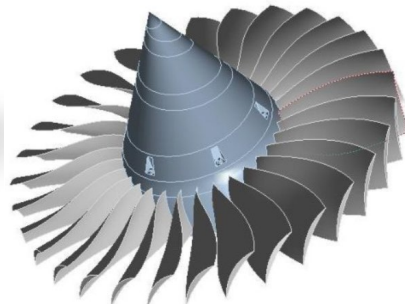
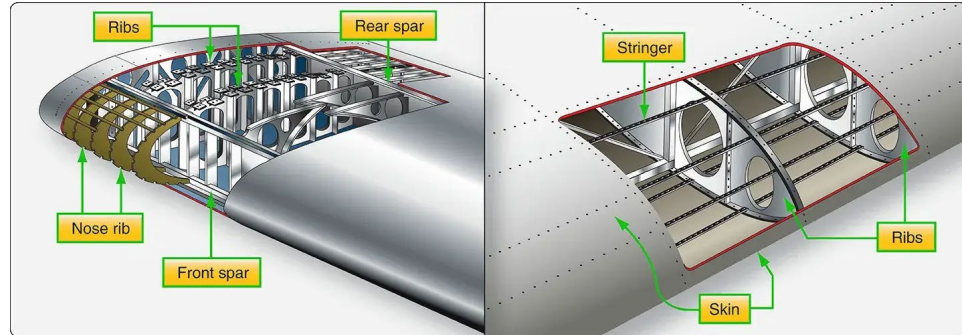
Reporter

Zixu XIA

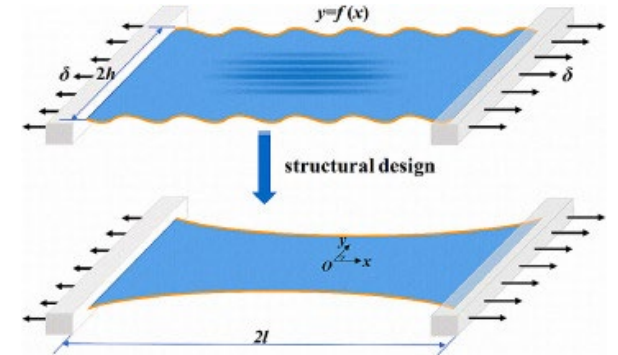
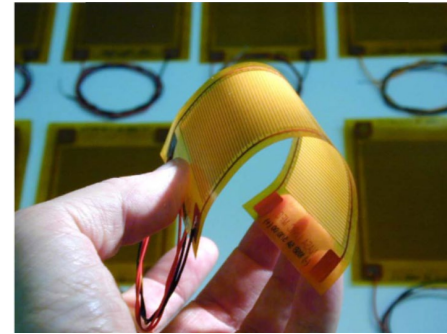
Supervisor

A/Prof. Yu CONG

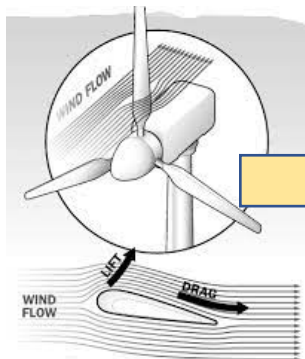
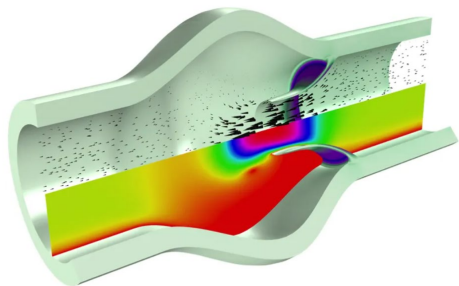
Engineering modeling



Thin films system



Fluid-structure interaction



Research highlights

1. How to establish an appropriate finite element method for modeling thin structures ?
2. An appropriate nonlinear dynamics reduced-order method to lower the cost of numerical simulations

Normal form method

- Touzé C. A **normal form approach** for nonlinear normal modes
- Vizzaccaro A, et al. **Direct computation** of nonlinear mapping via **normal form** for reduced-order models of finite element nonlinear structures

DPIM (Direct parametrisation of invariant manifold)

- Opreni A, et al. High-order **direct parametrisation of invariant manifolds** for model order reduction of finite element structures: application to generic forcing terms and parametrically excited systems
- Vizzaccaro A, et al. Direct parametrisation of invariant manifolds for generic **non-autonomous** systems including superharmonic resonances

Shell finite element modeling

- **Fewer degrees of freedom** compared to solid elements
- More convenient definition of the **kinematic description** of thin structures in the transverse direction
- Introduce **assumed natural strain** to prevent Poisson locking

Nonlinear dynamics solution methods

- Increment harmonic balance method (For **full order models**)
- Collocation method (For **reduced order models**)

These theories lay the foundation for our development of a framework for solving thin structures !



Positional relationship

$$\mathbf{X}(\theta^\alpha, \theta^3) = \mathbf{R}(\theta^\alpha) + \theta^3 \mathbf{a}_3(\theta^\alpha), \alpha = 1, 2$$

$$\mathbf{x}(\theta^\alpha, \theta^3) = \mathbf{r}(\theta^\alpha) + \theta^3 \tilde{\mathbf{a}}_3(\theta^\alpha), \alpha = 1, 2$$

Covariant base tensor

$$\mathbf{G}_\alpha = \mathbf{X}_{,\alpha} = \mathbf{a}_\alpha + \theta^3 \mathbf{a}_{3,\alpha} \quad \mathbf{g}_\alpha = \mathbf{x}_{,\alpha} = \tilde{\mathbf{a}}_\alpha + \theta^3 \tilde{\mathbf{a}}_{3,\alpha}$$

$$\mathbf{G}_3 = \mathbf{X}_3 = \mathbf{a}_3 \quad \mathbf{g}_3 = \mathbf{x}_3 = \tilde{\mathbf{a}}_3$$

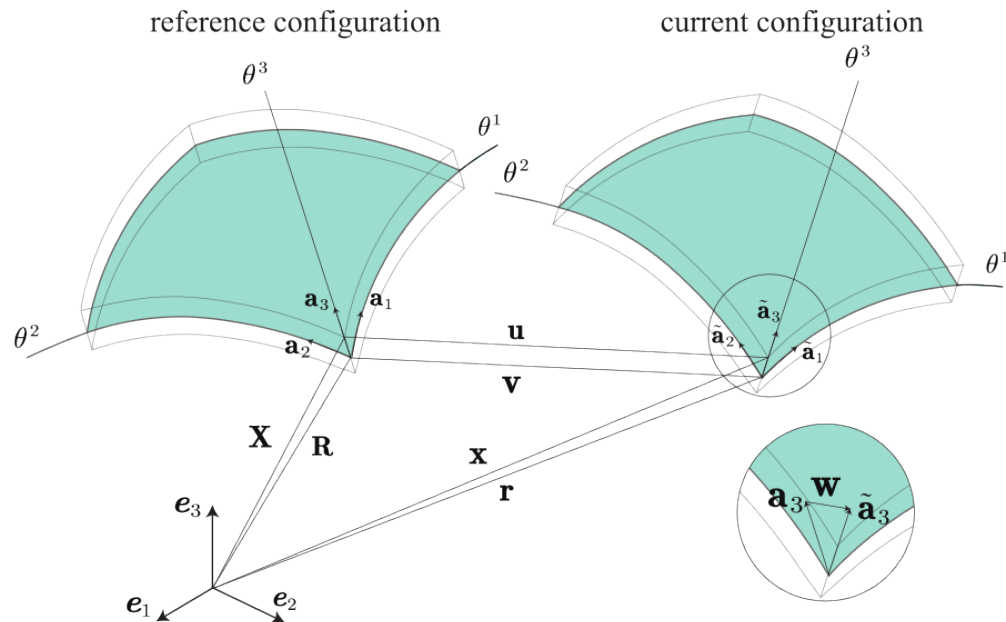
Green-Lagrange strain

$$\begin{cases} E_{ij} = \frac{1}{2} (\mathbf{g}_i \cdot \mathbf{g}_j - \mathbf{G}_i \cdot \mathbf{G}_j) \\ E_{ij} = \frac{1}{2} \left(\mathbf{G}_i \cdot \frac{\partial \mathbf{u}}{\partial \theta^j} + \mathbf{G}_j \cdot \frac{\partial \mathbf{u}}{\partial \theta^i} + \frac{\partial \mathbf{u}}{\partial \theta^i} \cdot \frac{\partial \mathbf{u}}{\partial \theta^j} \right) \end{cases}$$

Constitutive relation

$$\mathbf{S} = \mathbb{D} : \mathbf{E} \quad \text{Traditional shell elements lead to locking issues}$$

$$E_{33}^{(0)} + \theta^3 E_{33}^{(1)} \simeq - \frac{D^{33ij}}{D^{3333}} (E_{ij}^{(0)} + \theta^3 E_{ij}^{(1)})$$



Enhanced assumed strain

$$\mathbf{E}^{\text{full}} = \mathbf{E} + \tilde{\mathbf{E}} \quad \text{with} \quad \int_{\Omega} \mathbf{S} : \delta \tilde{\mathbf{E}} d\Omega = 0$$

Hu-Washizu functional

$$\Pi_{HW}(\mathbf{u}, \mathbf{E}^{\text{full}}, \mathbf{S}) = \int_{\Omega} \frac{1}{2} \rho \dot{\mathbf{u}} \cdot \dot{\mathbf{u}} d\Omega + \int_{\Omega} \frac{1}{2} \mathbf{E}^{\text{full}} : \mathbb{D} : \mathbf{E}^{\text{full}} d\Omega$$

$$- \int_{\Omega} \mathbf{S} : (\mathbf{E}^{\text{full}} - \mathbf{E}) d\Omega - \mathbf{F}(t) \cdot \mathbf{u}$$

$$\begin{cases} \int_{\Omega} \rho \ddot{\mathbf{u}} \cdot \delta \mathbf{u} d\Omega + \int_{\Omega} \mathbf{S} : \delta \mathbf{E} d\Omega - \mathbf{F}(t) \cdot \delta \mathbf{u} = 0 \\ \int_{\Omega} \mathbf{S} : \delta \tilde{\mathbf{E}} d\Omega = 0 \end{cases}$$

Traditional shell elements

Additional equations

Extend traditional variational equations to avoid locking issues

Shape function interpolation

$$\{\mathbf{u}\} = [\mathbf{N}] \{\mathbf{u}^{(e)}\}, \quad \{\delta \mathbf{u}\} = [\mathbf{N}] \{\delta \mathbf{u}^{(e)}\}, \quad \{\nabla \mathbf{u}\} = [\mathbf{\Xi}] \{\mathbf{u}^{(e)}\}$$

Green-Lagrange strain

$$\{\mathbf{E}\} = \left([\mathbf{B}_l] + \frac{1}{2} [\mathbf{B}_{nl}(\mathbf{u})] \right) \{\mathbf{u}^{(e)}\} = \left([\mathbf{R}] + \frac{1}{2} [\mathbf{A}(\mathbf{u})] \right) [\mathbf{\Xi}] \{\mathbf{u}^{(e)}\}$$

$$\{\delta \mathbf{E}\} = \left([\mathbf{B}_l] + [\mathbf{B}_{nl}(\mathbf{u})] \right) \{\mathbf{u}^{(e)}\} = \left([\mathbf{R}] + [\mathbf{A}(\mathbf{u})] \right) [\mathbf{\Xi}] \{\delta \mathbf{u}^{(e)}\}$$

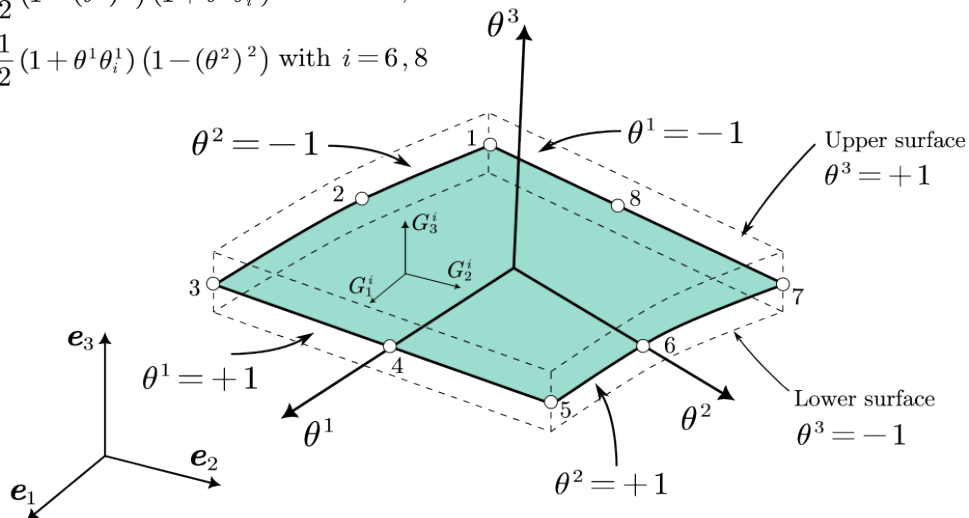
Interpolation of enhanced assumed strain

$$\{\tilde{\mathbf{E}}\} = [\mathbf{B}_\alpha] \{\alpha^{(e)}\} \quad \text{with} \quad \tilde{E}_{33} = \alpha_1 + \alpha_2 \theta^1 + \alpha_3 \theta^2 + \alpha_4 \theta^1 \theta^2$$

$$N^i = \frac{1}{4} (1 + \theta^1 \theta_i^1) (1 + \theta^2 \theta_i^2) (\theta^1 \theta_i^1 + \theta^2 \theta_i^2 - 1) \quad \text{with } i = 1, 2, 3, 4$$

$$N^i = \frac{1}{2} (1 - (\theta^1)^2) (1 + \theta^2 \theta_i^2) \quad \text{with } i = 5, 7$$

$$N^i = \frac{1}{2} (1 + \theta^1 \theta_i^1) (1 - (\theta^2)^2) \quad \text{with } i = 6, 8$$



Development of the dynamic equation

$$[\mathbf{M}] \{\ddot{\mathbf{U}}\} + [\mathbf{C}] \{\dot{\mathbf{U}}\} + [\mathbf{K}_l] \{\mathbf{U}\} + \{\mathbf{G}(\mathbf{U}, \mathbf{U})\} + \{\mathbf{H}(\mathbf{U}, \mathbf{U}, \mathbf{U})\} = \{\mathbf{F}(t)\}$$

where

$$[\mathbf{M}] = \bigwedge_{k=1}^N \int_{\Omega} \rho [\mathbf{N}]^T [\mathbf{N}] d\Omega$$

$$[\mathbf{K}_l] = \bigwedge_{k=1}^N \left(\int_{\Omega} [\mathbf{B}_l]^T [\mathbf{D}] [\mathbf{B}_l] d\Omega - \int_{\Omega} [\mathbf{B}_l]^T [\mathbf{D}] [\mathbf{B}_\alpha] d\Omega \times [\mathbf{k}_{\alpha\alpha}]^{-1} \times \int_{\Omega} [\mathbf{B}_\alpha]^T [\mathbf{D}] [\mathbf{B}_l] d\Omega \right)$$

$$\begin{aligned} \{\mathbf{G}(\mathbf{U}, \mathbf{U})\} = & \bigwedge_{k=1}^N \left(\int_{\Omega} [\mathbf{B}_{nl}(\mathbf{u})]^T [\mathbf{D}] [\mathbf{B}_l] d\Omega - \frac{1}{2} \int_{\Omega} [\mathbf{B}_l]^T [\mathbf{D}] [\mathbf{B}_{nl}(\mathbf{u})] d\Omega \right. \\ & - \frac{1}{2} \int_{\Omega} [\mathbf{B}_l]^T [\mathbf{D}] [\mathbf{B}_\alpha] d\Omega \times [\mathbf{k}_{\alpha\alpha}]^{-1} \times \int_{\Omega} [\mathbf{B}_\alpha]^T [\mathbf{D}] [\mathbf{B}_{nl}(\mathbf{u})] d\Omega \\ & \left. - \int_{\Omega} [\mathbf{B}_{nl}(\mathbf{u})]^T [\mathbf{D}] [\mathbf{B}_\alpha] d\Omega \times [\mathbf{k}_{\alpha\alpha}]^{-1} \times \int_{\Omega} [\mathbf{B}_\alpha]^T [\mathbf{D}] [\mathbf{B}_l] d\Omega \right) \{\mathbf{u}^{(e)}\} \end{aligned}$$

$$\begin{aligned} \{\mathbf{H}(\mathbf{U}, \mathbf{U}, \mathbf{U})\} = & \bigwedge_{k=1}^N \left(\frac{1}{2} \int_{\Omega} [\mathbf{B}_{nl}(\mathbf{u})]^T [\mathbf{D}] [\mathbf{B}_{nl}(\mathbf{u})] d\Omega \right. \\ & \left. - \frac{1}{2} \int_{\Omega} [\mathbf{B}_{nl}(\mathbf{u})]^T [\mathbf{D}] [\mathbf{B}_\alpha] d\Omega \times [\mathbf{k}_{\alpha\alpha}]^{-1} \times \int_{\Omega} [\mathbf{B}_\alpha]^T [\mathbf{D}] [\mathbf{B}_{nl}(\mathbf{u})] d\Omega \right) \{\mathbf{u}^{(e)}\} \end{aligned}$$

$$[\mathbf{C}] = p_1 [\mathbf{M}] + p_2 [\mathbf{K}_l]$$

Establish standard nonlinear dynamic equations to address DPIM solutions

Second-order dynamic equation

$$[\mathbf{M}]\{\ddot{\mathbf{U}}\} + [\mathbf{C}]\{\dot{\mathbf{U}}\} + [\mathbf{K}_l]\{\mathbf{U}\} + \{\mathbf{G}(\mathbf{U}, \mathbf{U})\} + \{\mathbf{H}(\mathbf{U}, \mathbf{U}, \mathbf{U})\} = \{\mathbf{F}(t)\} \quad \longrightarrow \quad [\mathbf{B}]\{\dot{\mathbf{y}}\} = [\mathbf{A}]\{\mathbf{y}\} + \{\mathbf{Q}(\mathbf{y}, \mathbf{y})\} + \{\boldsymbol{\Upsilon}\}\tilde{\mathbf{z}} \quad \text{and} \quad \dot{\tilde{\mathbf{z}}} = \tilde{\lambda}\tilde{\mathbf{z}}$$

Nonlinear mapping

$$\{\mathbf{y}\} = \mathbf{W}(\mathbf{z})$$

ROM equations

$$\{\dot{\mathbf{z}}\} = \mathbf{f}(\mathbf{z})$$



$$\mathbf{W}(\mathbf{z}) = \sum_{p=1}^o \langle \mathbf{W}(\mathbf{z}) \rangle_p$$

$$\mathbf{f}(\mathbf{z}) = \sum_{p=1}^o \langle \mathbf{f}(\mathbf{z}) \rangle_p$$



$$[\mathbf{B}] \left\langle \frac{\partial \mathbf{W}(\mathbf{z})}{\partial \mathbf{z}} \mathbf{f}(\mathbf{z}) \right\rangle_p = [\mathbf{A}] \langle \mathbf{W}(\mathbf{z}) \rangle_p + \langle \mathbf{Q}(\mathbf{W}(\mathbf{z}), \mathbf{W}(\mathbf{z})) \rangle_p + \langle \boldsymbol{\Upsilon} \tilde{\mathbf{z}} \rangle_p$$

$$[\mathbf{B}] \left\langle \frac{\partial \mathbf{W}(\mathbf{z})}{\partial \mathbf{z}} \mathbf{f}(\mathbf{z}) \right\rangle_1 = [\mathbf{A}] \langle \mathbf{W}(\mathbf{z}) \rangle_1 + \langle \boldsymbol{\Upsilon} \tilde{\mathbf{z}} \rangle_1 \quad p = 1$$

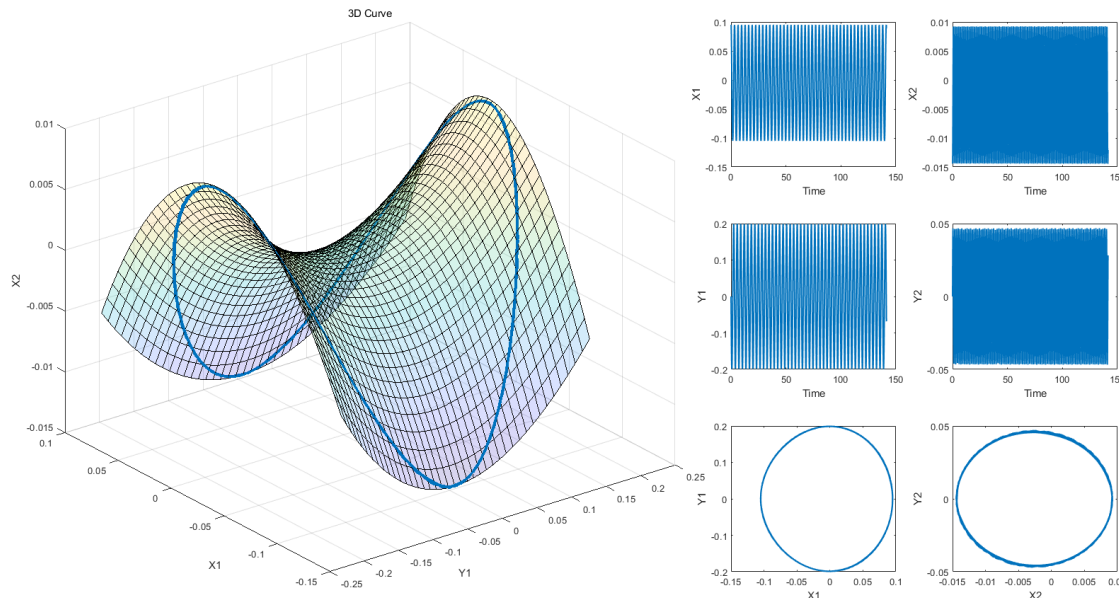
$$[\mathbf{B}] \left\langle \frac{\partial \mathbf{W}(\mathbf{z})}{\partial \mathbf{z}} \mathbf{f}(\mathbf{z}) \right\rangle_p = [\mathbf{A}] \langle \mathbf{W}(\mathbf{z}) \rangle_p + \langle \mathbf{Q}(\mathbf{W}(\mathbf{z}), \mathbf{W}(\mathbf{z})) \rangle_p \quad p > 1$$

Order-1 homological equation

$$\begin{bmatrix} \tilde{\lambda} \mathbf{B} - \mathbf{A} & \mathbf{B} \Phi_{\mathcal{R}} & 0 \\ \Phi_{\mathcal{R}}^{\dagger} \mathbf{B} & 0 & 0 \\ 0 & 0 & \mathbf{I} \end{bmatrix} \begin{bmatrix} \mathbf{W}^{(1,d+1)} \\ \mathbf{f}_{\mathcal{R}}^{(1,d+1)} \\ \mathbf{f}_{\mathcal{R}}^{(1,d+1)} \end{bmatrix} = \begin{bmatrix} \boldsymbol{\Upsilon} \\ 0 \\ 0 \end{bmatrix}$$

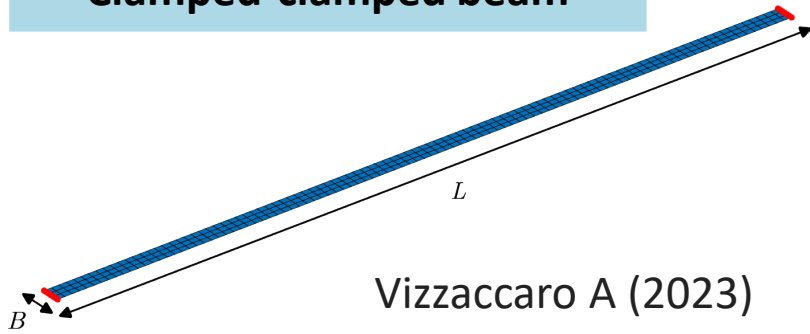
Order-p homological equation

$$\begin{bmatrix} \sigma^{(p,k)} \mathbf{B} - \mathbf{A} & \mathbf{B} \Phi_{\mathcal{R}} & 0 \\ \Phi_{\mathcal{R}}^{\dagger} \mathbf{B} & 0 & 0 \\ 0 & 0 & \mathbf{I} \end{bmatrix} \begin{bmatrix} \mathbf{W}^{(p,k)} \\ \mathbf{f}_{\mathcal{R}}^{(p,k)} \\ \mathbf{f}_{\mathcal{R}}^{(p,k)} \end{bmatrix} = \begin{bmatrix} \mathbf{R}^{(p,k)} \\ 0 \\ 0 \end{bmatrix}$$



Numerical example: modal analysis

Clamped-clamped beam



Geometric parameters

$$L = 1000 \mu\text{m}$$

$$B = 24 \mu\text{m}$$

$$H = 10 \mu\text{m}$$

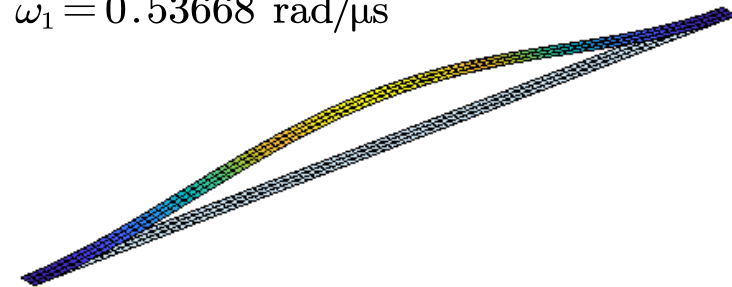
Material parameters

$$E = 160 \text{ Gpa}$$

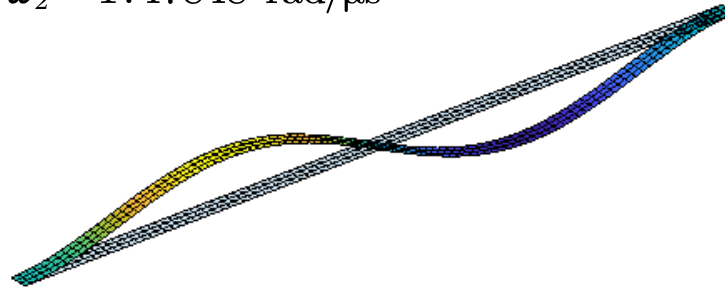
$$\nu = 0.22$$

$$\rho = 2320 \text{ kg/m}^3$$

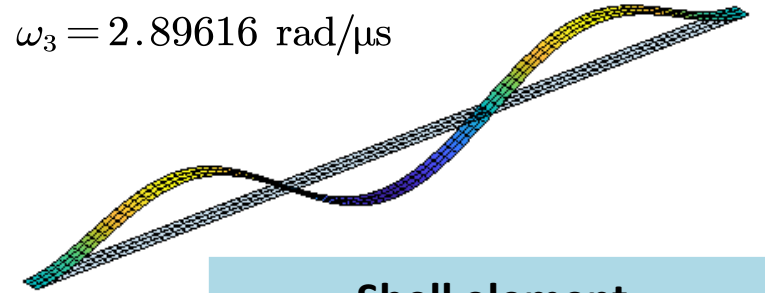
$$\omega_1 = 0.53668 \text{ rad}/\mu\text{s}$$



$$\omega_2 = 1.47848 \text{ rad}/\mu\text{s}$$

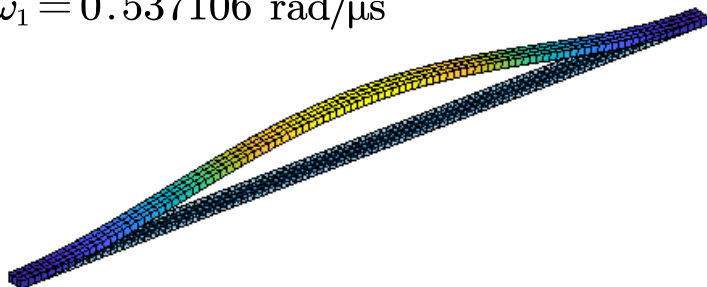


$$\omega_3 = 2.89616 \text{ rad}/\mu\text{s}$$

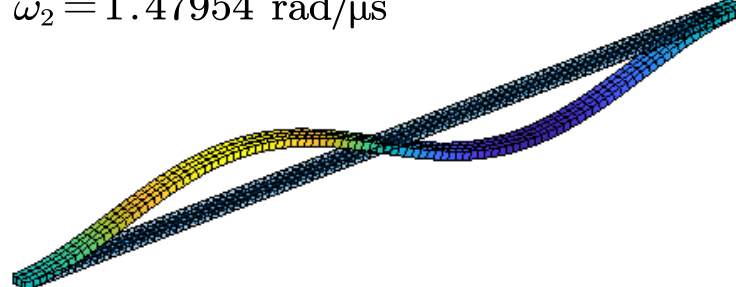


Shell element

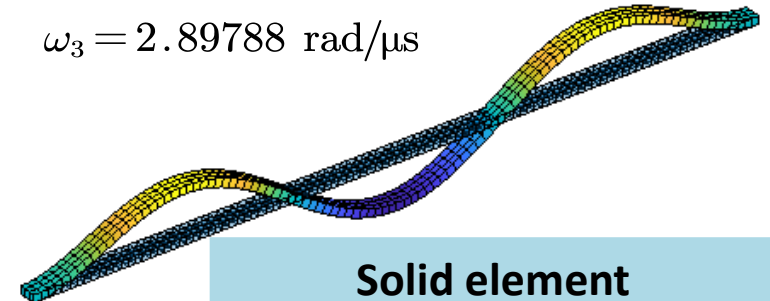
$$\omega_1 = 0.537106 \text{ rad}/\mu\text{s}$$



$$\omega_2 = 1.47954 \text{ rad}/\mu\text{s}$$



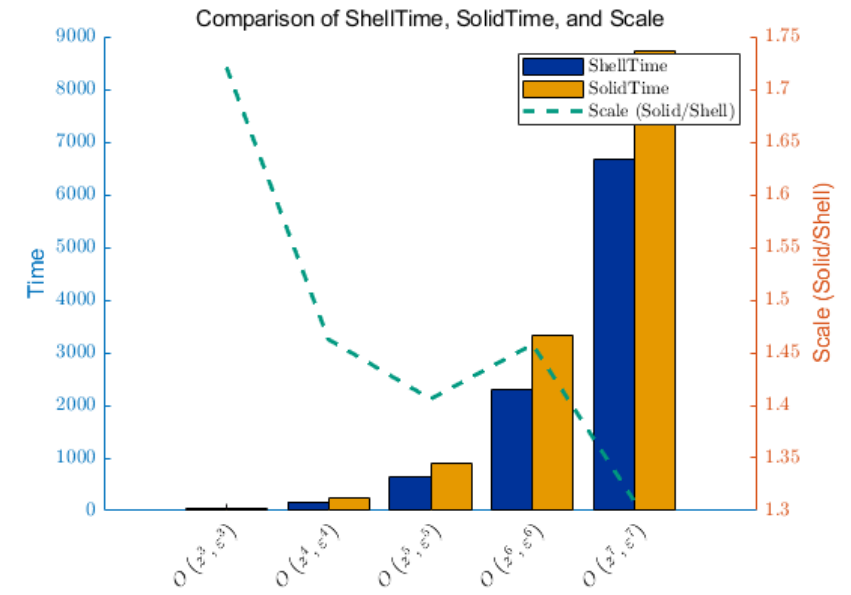
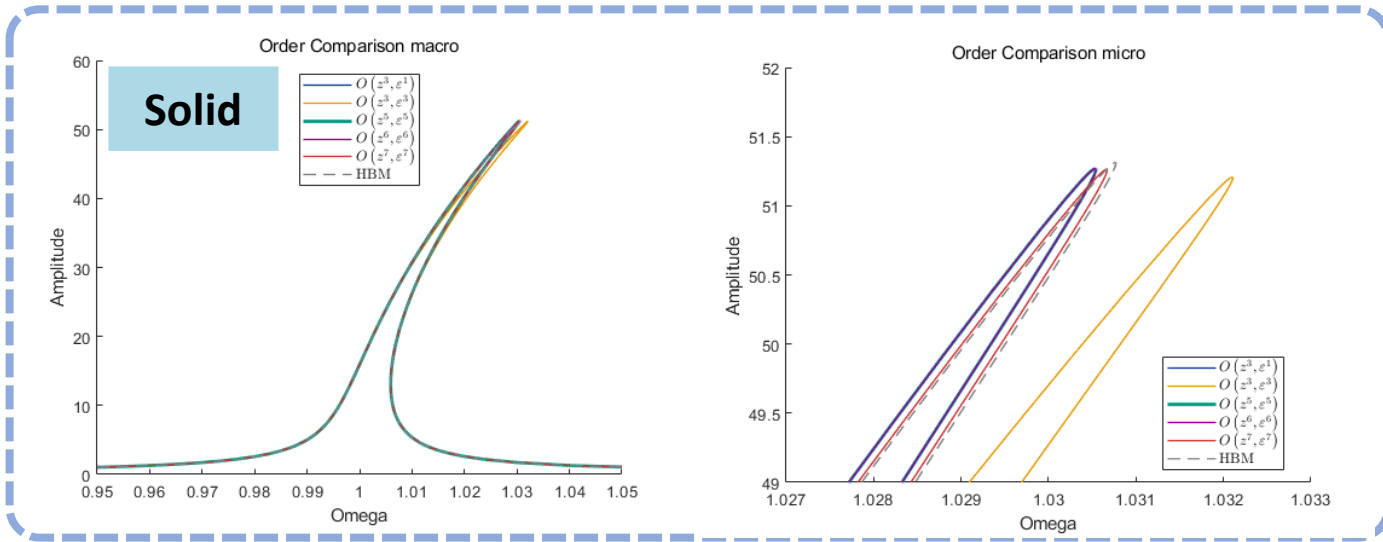
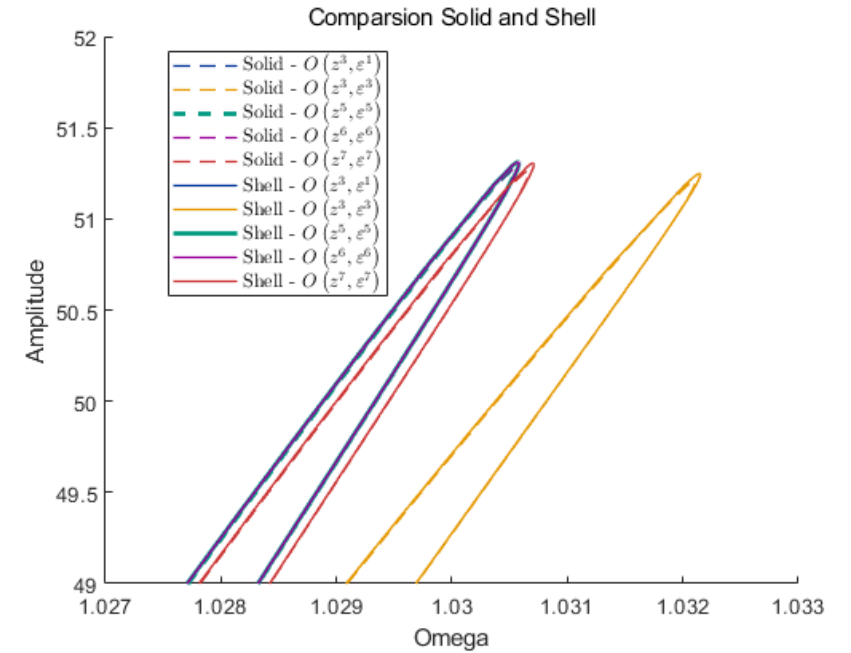
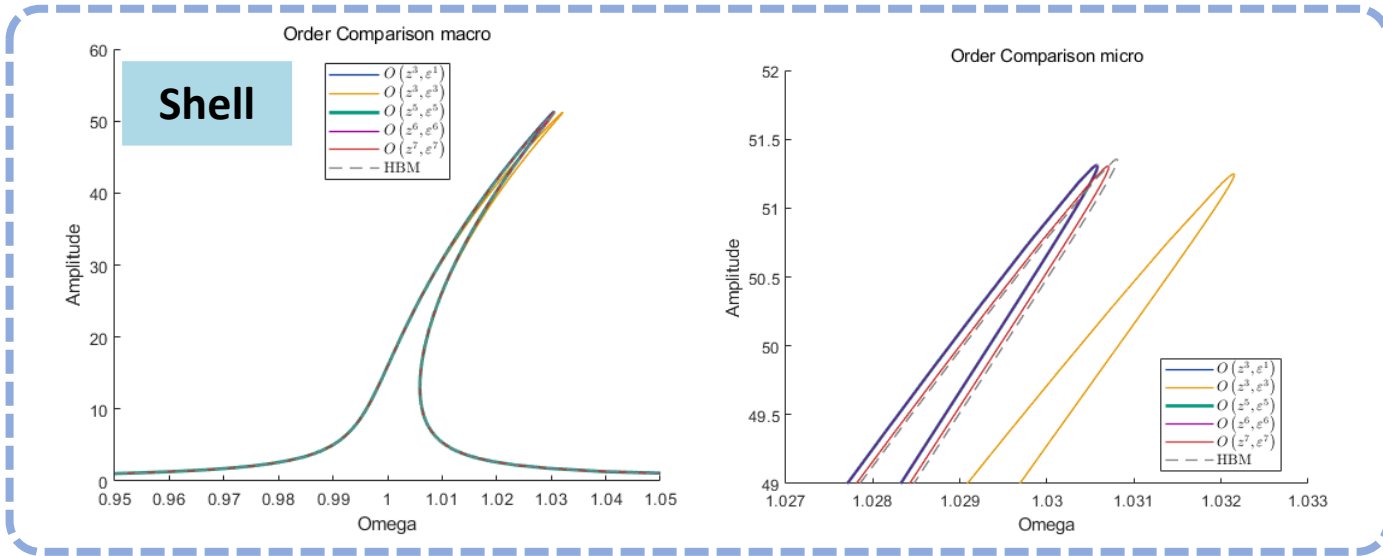
$$\omega_3 = 2.89788 \text{ rad}/\mu\text{s}$$



Solid element

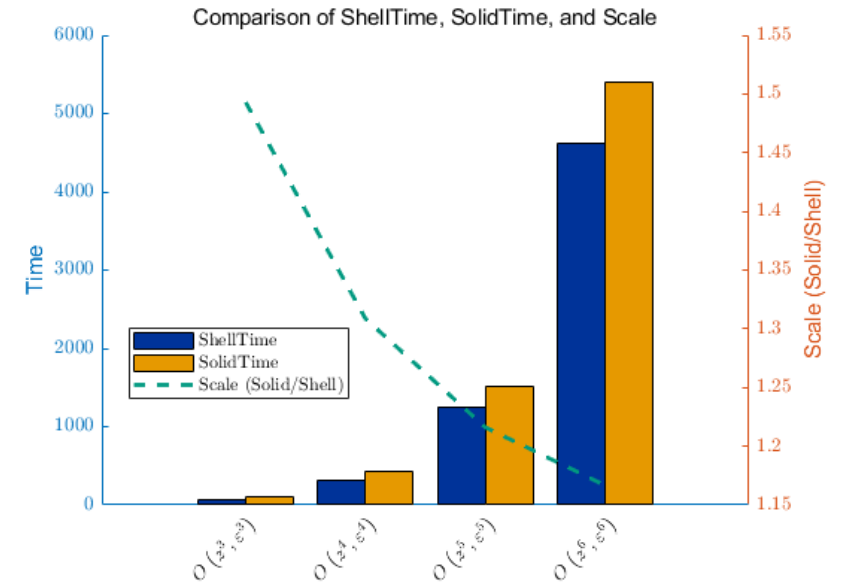
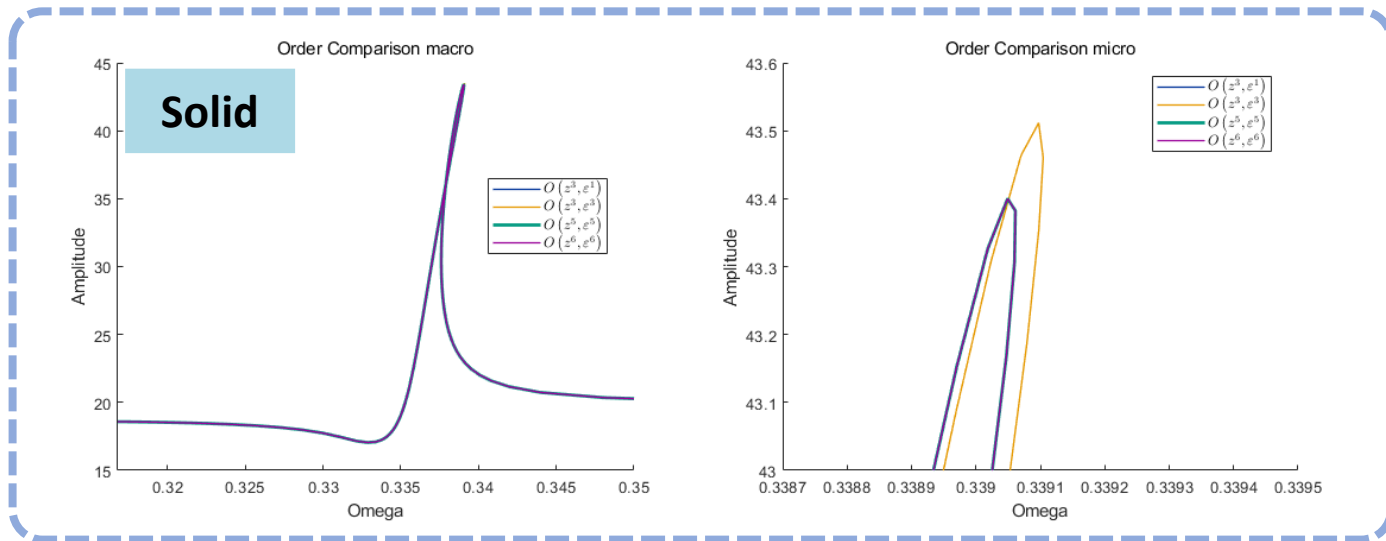
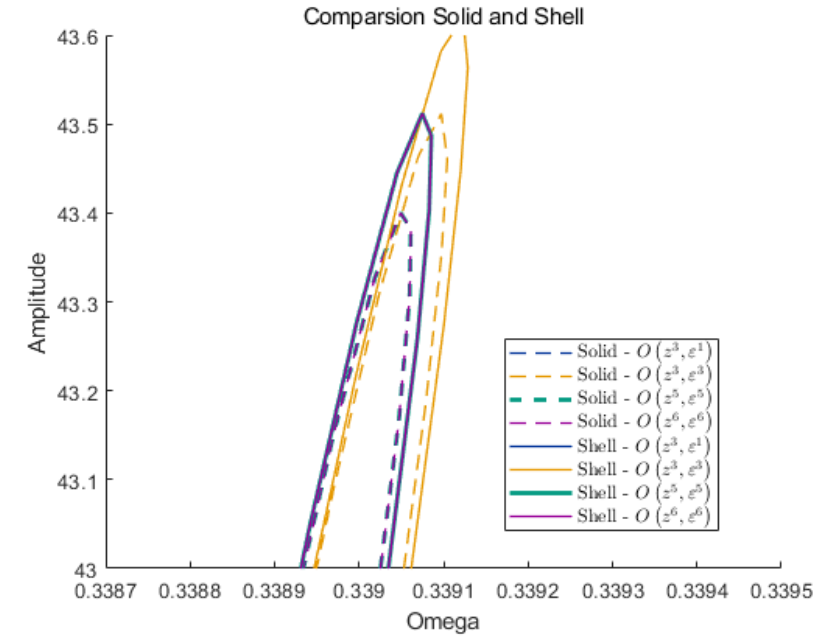
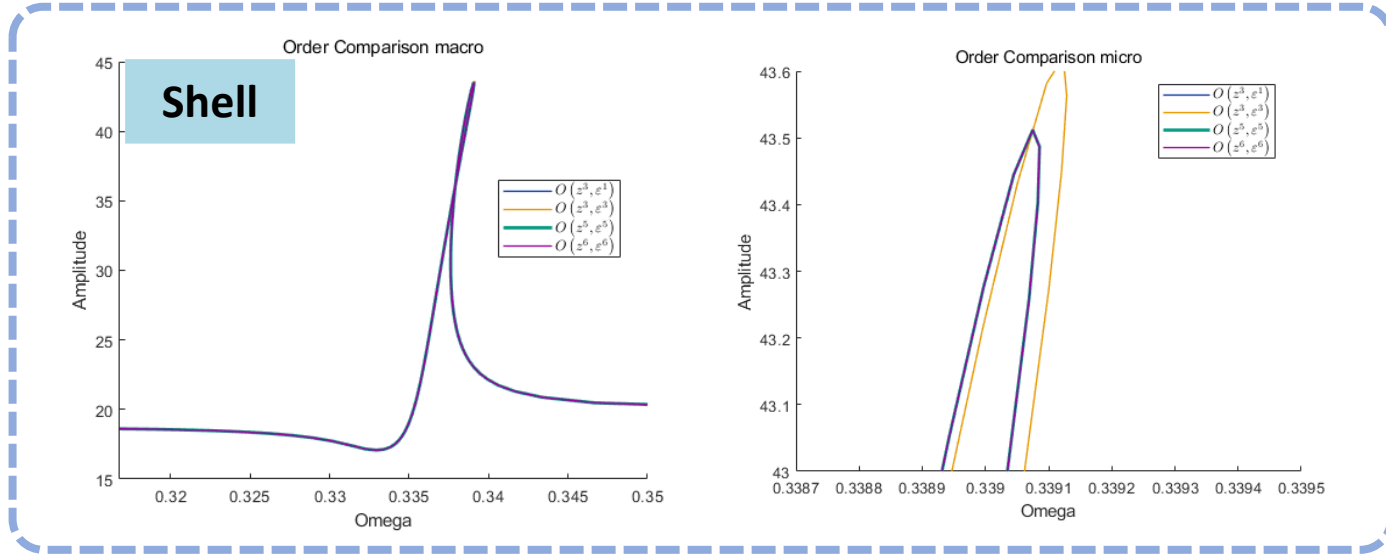
Numerical example: primary mode

$$[\mathbf{M}]\{\ddot{\mathbf{U}}\} + [\mathbf{C}]\{\dot{\mathbf{U}}\} + [\mathbf{K}_l]\{\mathbf{U}\} + \{\mathbf{G}(\mathbf{U}, \mathbf{U})\} + \{\mathbf{H}(\mathbf{U}, \mathbf{U}, \mathbf{U})\} = 0.03[\mathbf{M}]\{\Phi_1\}\cos\Omega t$$



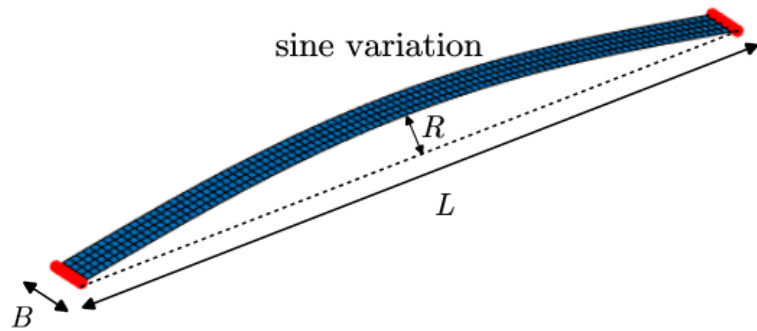
Numerical example: 1:3 superharmonic resonance

$$[\mathbf{M}]\{\ddot{\mathbf{U}}\} + [\mathbf{C}]\{\dot{\mathbf{U}}\} + [\mathbf{K}_l]\{\mathbf{U}\} + \{\mathbf{G}(\mathbf{U}, \mathbf{U})\} + \{\mathbf{H}(\mathbf{U}, \mathbf{U}, \mathbf{U})\} = 5[\mathbf{M}]\{\Phi_1\}\cos\Omega t$$



Numerical example: 1:2 superharmonic resonance

Clamped-clamped curve beam



Geometric parameters

$$L = 640 \text{ } \mu\text{m}$$

$$B = 32 \text{ } \mu\text{m}$$

$$R = 3.84 \text{ } \mu\text{m}$$

$$H = 6.4 \text{ } \mu\text{m}$$

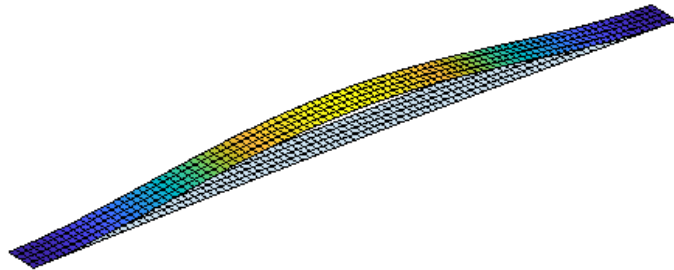
Material parameters

$$E = 160 \text{ GPa}$$

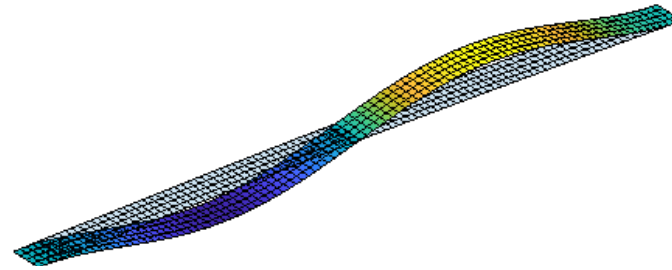
$$\nu = 0.22$$

$$\rho = 2320 \text{ kg/m}^3$$

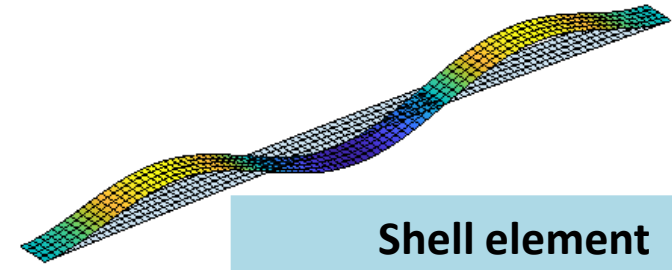
$$\omega_1 = 0.996082 \text{ rad}/\mu\text{s}$$



$$\omega_2 = 2.31348 \text{ rad}/\mu\text{s}$$

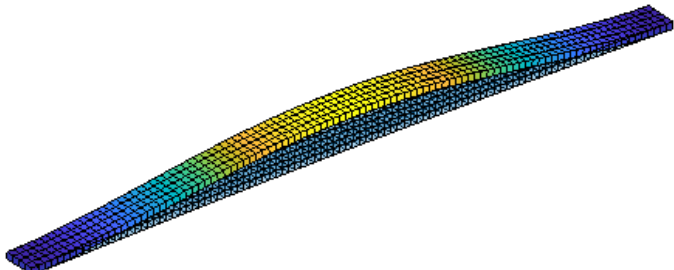


$$\omega_3 = 4.53315 \text{ rad}/\mu\text{s}$$

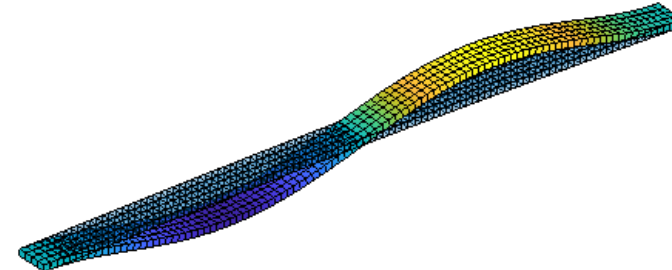


Shell element

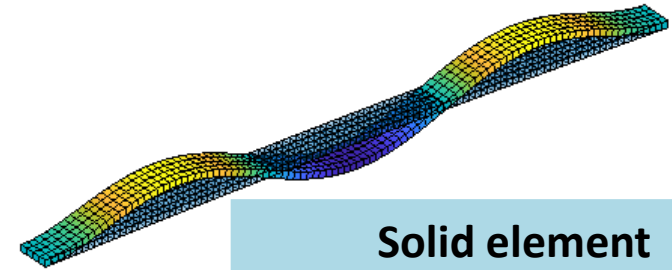
$$\omega_1 = 0.996442 \text{ rad}/\mu\text{s}$$



$$\omega_2 = 2.31507 \text{ rad}/\mu\text{s}$$



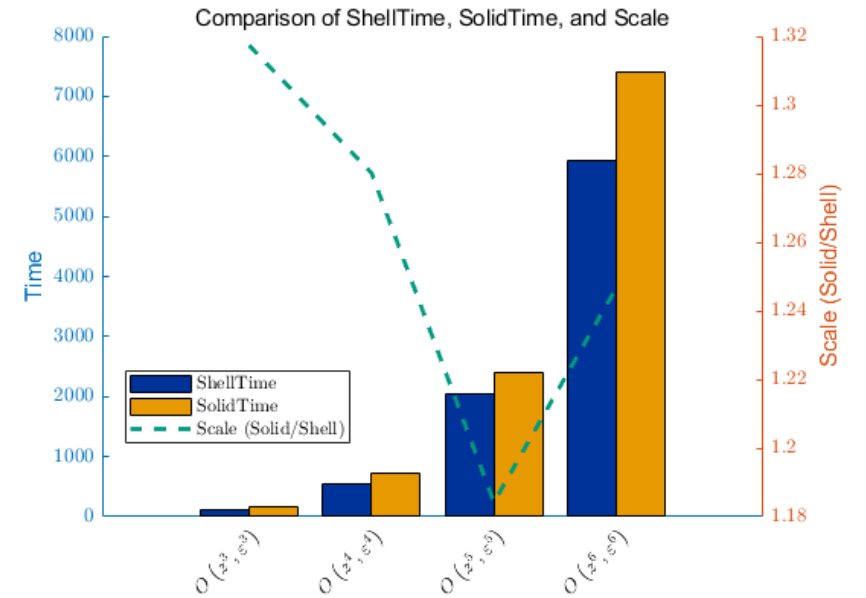
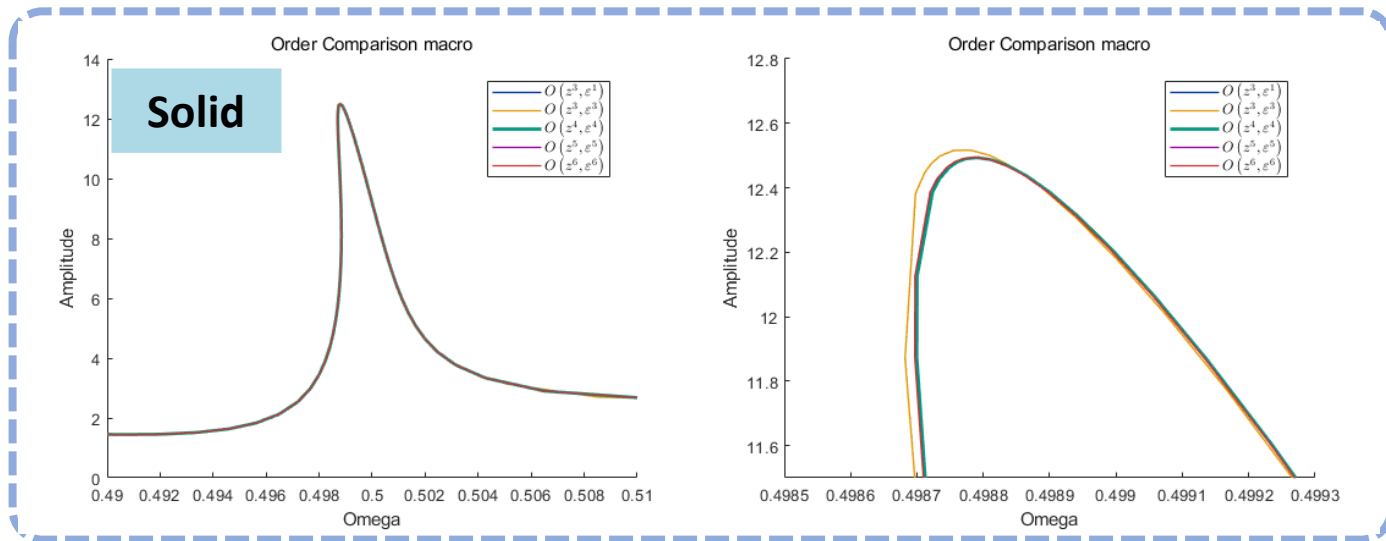
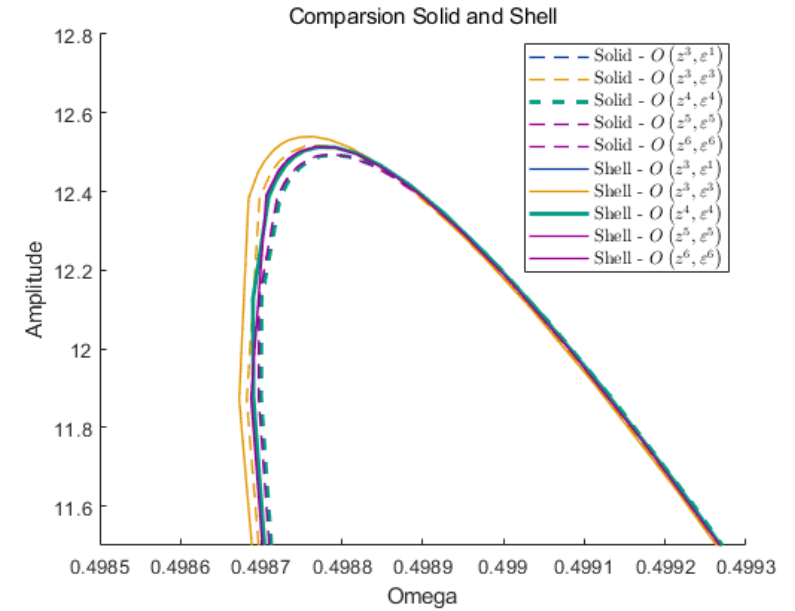
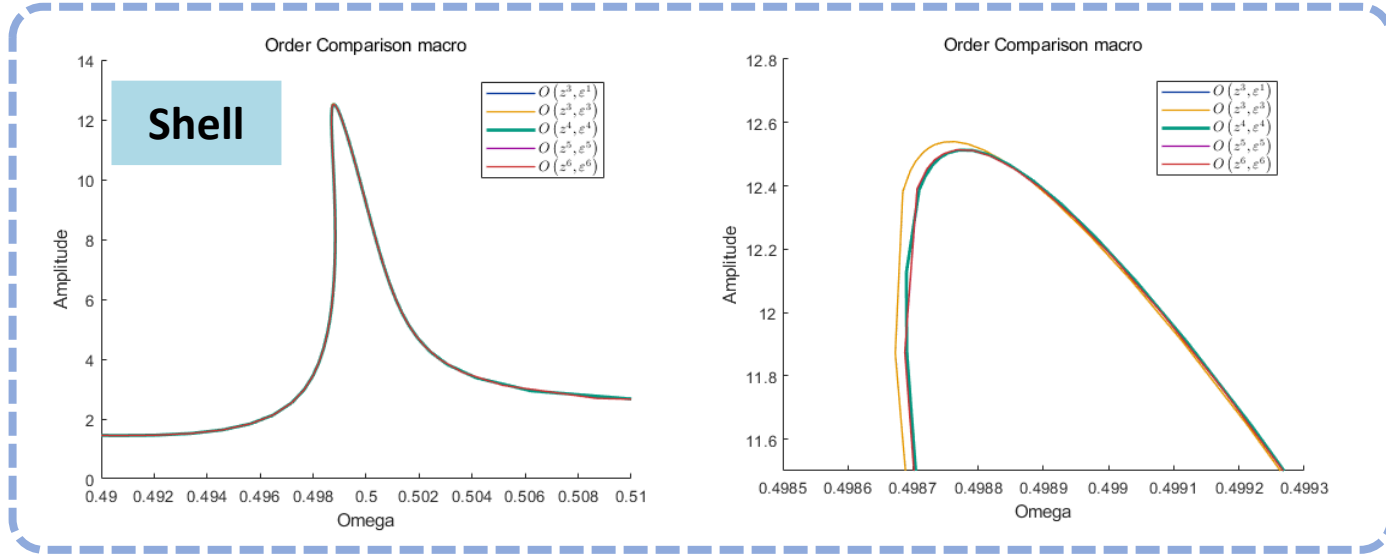
$$\omega_3 = 4.53592 \text{ rad}/\mu\text{s}$$



Solid element

Numerical example: 1:2 superharmonic resonance

$$[M]\{\ddot{U}\} + [C]\{\dot{U}\} + [K_r]\{U\} + \{G(U, U)\} + \{H(U, U, U)\} = 1.5 [M] \{\Phi_1\} \cos \Omega t$$



Highlights

1. Established a **shell element model** based on DPIM for application in reduced-order methods for nonlinear dynamics of thin structures
2. By comparing with solid elements and full-order model computation methods, we verified the **effectiveness of the shell element** developed in this work in reduced-order methods
3. Compared to solid elements, the **computational cost is lower**
4. The **kinematic assumptions** of thin structures can be conveniently applied in the thickness direction

Future works

1. Based on the validation results of this work, we will proceed to study more **practical engineering problems** concerning complex nonlinear geometrically thin structures, which are not convenient to analyze using solid elements
2. On this basis, we will also consider more complex working conditions, including **fluid-structure interaction** problems and **material nonlinearity** issues

Thanks for your listening

17-18 octobre 2024

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